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TITLE: Amendment 1 – Wind energy generation systems – Part 1: Design requirements

PROPOSED STABILITY DATE: 2025

NOTE FROM TC/SC OFFICERS:

CONTENTS

FOREWORD.....	3
1 Normative references	5
2 Terms and definitions	5
3 Symbols and abbreviated terms.....	5
6.3 Wind conditions	6
6.3.1 General.....	6
6.3.3.2 Extreme wind speed model (EWM)	8
7.4.1 General.....	8
7.4.7 Parked (standstill or idling) (DLC 6.1 to 6.4).....	8
7.6.1.3 Partial safety factor for consequences of failure and component classes	9
7.6.2.2 Partial safety factors for loads.....	9
7.6.2.4 Partial safety factors for resistances where recognized design codes are not available	9
7.6.2.5 Partial safety factors for materials where recognized design codes are available	9
7.6.3.4 Partial material factors where recognized design codes are available	9
10.12 Electro Magnetic Compatibility (EMC).....	11
10.12.1 Introduction	11
10.12.2 EMC design requirements.....	11
11.3.2 Wind condition parameters	12
11.3.4 Data evaluation.....	12
11.9.2 Assessment of the fatigue load suitability by reference to wind data	12
11.9.3 Assessment of the ultimate load suitability by reference to wind data.....	13
11.10 Assessment of structural integrity by load calculations with reference to site-specific conditions	14
Annex B.....	14
B.1 General.....	14
B.2 Power production (DLC 1.1 to 1.9).....	15
Annex E.....	15
L.2 Ice mass effects on wind turbine blades	16
L.4 Reference documents and bibliography	17
Annex N.....	17
N.1 General.....	17
N.2 Reference documents and bibliography.....	18

INTERNATIONAL ELECTROTECHNICAL COMMISSION

WIND TURBINES –

Part 1: Design Requirements

AMENDMENT 1

FOREWORD

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Amendment 1 to IEC 61400-1:2019 has been prepared by IEC technical committee 88: Wind energy generation systems.

The text of this Amendment is based on the following documents:

Draft	Report on voting
XX/XX/XXXX	XX/XX/XXX

Full information on the voting for its approval can be found in the report on voting indicated in the above table.

The language used for the development of this Amendment is English.

This document was drafted in accordance with ISO/IEC Directives, Part 2, and developed in accordance with ISO/IEC Directives, Part 1 and ISO/IEC Directives, IEC Supplement, available at www.iec.ch/members_experts/refdocs. The main document types developed by IEC are described in greater detail at www.iec.ch/standardsdev/publications/.

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- reconfirmed,
- withdrawn,
- replaced by a revised edition, or
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1 Normative references

2 Terms and definitions

For the purposes of this document, the terms and definitions given in IEC 61400-1:2019 and the following apply.

ISO and IEC maintain terminological databases for use in standardization at the following addresses:

- IEC Electropedia: available at <http://www.electropedia.org/>
- ISO Online browsing platform: available at <http://www.iso.org/obp>

Add

damage equivalent load

Constant amplitude load derived from the load spectrum and a given S-N curve exponent that results in an equivalent fatigue damage

reference loads

The loads that had been utilised for detailed structural verification of the wind turbine components are called reference loads.

serviceability

ability of a structure or structural element to perform adequately for normal use under all expected actions

serviceability limit state

state which corresponds to conditions beyond which specified service requirements for a structure or structural element are no longer met

S1 - SLS characteristic load

serviceability limit state load level equal to the characteristic value of the loads from ultimate limit states classified as N (Normal)

S2 - SLS 10⁻⁴ frequent load case

serviceability limit state load level for frequent actions, which are exceeded for 10⁻⁴ of the lifetime,

S3 - SLS 10⁻² frequent load case

serviceability limit state load level for the equivalent to frequent actions, which are exceeded for 10⁻² of the lifetime.

3 Symbols and abbreviated terms

4.2 Abbreviated terms

Add

DEL Damage equivalent load, S_{eq} , determined from the approach that it leads to the same damage for a given reference number of load cycles, n_{eq} , as the real load spectrum under the assumption that the damage can be determined on basis of the load cycles from a linear S-N curve with a given slope, m . Let the discrete load spectrum be specified by the number of cycles n_i for the load S_i , $i = 1, 2, \dots, n_S$. Then the equivalent load can be calculated from the equation

$$S_{eq} = \left(\frac{\sum_{i=1}^{n_S} n_i S_i^m}{n_{eq}} \right)^{1/m}$$

42

43 EM Electro Magnetic

44 EMC Electro Magnetic Compatibility

45 NTM90 Normal Turbulence Model, representative value of 90% percentile value of
46 distribution47 **6.2 Wind turbine class**48 *Replace paragraph 2 with*

49 Class T assumes all wind model parameters to be the same and allows the combination of $V_{ref,T}$ with
50 all turbulence categories. It does not cover all the areas prone to tropical cyclones. **The evaluation of
51 the 1-year return period storm wind speed should be done independently of the 50-year return period
52 storm.** A site assessment based on Clause 11 is needed, as a minimum assessing that V_{50} is below V_{ref}
53 of class T ($V_{ref,T}$), **and that V_1 is below the classification value.**

54 **6.3 Wind conditions**55 **6.3.1 General**56 *Replace paragraphs 3 and 4 with*

57 The wind regime for load and safety considerations is divided into the normal wind conditions,
58 which will occur frequently during normal operation of a wind turbine, and the extreme wind
59 conditions that are defined as having a 1-year or 50-year return period.¹

60 The wind conditions include a constant mean flow combined, in many cases, with either a
61 varying deterministic gust profile or with turbulence. In all cases, an upwards inclination of the
62 mean flow with respect to a horizontal plane of 8° shall be considered. This flow inclination
63 angle shall be assumed to be invariant with height.

64 **6.3.2.3 Normal turbulence model (NTM)**65 *Replace the clause with*

66 For the normal turbulence model, the turbulence standard deviation, σ_1 , shall be defined for the
67 standard wind turbine classes based on the Weibull distribution in equation (10) for the given
68 hub height wind speed.

69 The Weibull distribution for σ_1 shall either be applied as a distribution with scale and shape
70 parameters as in equation (11) or by the 90% quantile value in equation (12)²:

$$R_W(\sigma_1 < \sigma_0) = 1 - \exp \left[- \left(\frac{\sigma_0}{C} \right)^k \right] \quad (1)$$

¹ The return period of the extreme event is independent of the design lifetime of the turbine as the largest value for the normal failure probability is given for a single year (see Annex K)

² The choice of NTM model affects the level of reliability against fatigue failure. Using the Weibull distribution is more robust for inclusion of non-linear effects, but the resulting fatigue loads have no bias and therefore result in a lower reliability level in most cases compared to using the 90% quantile value.

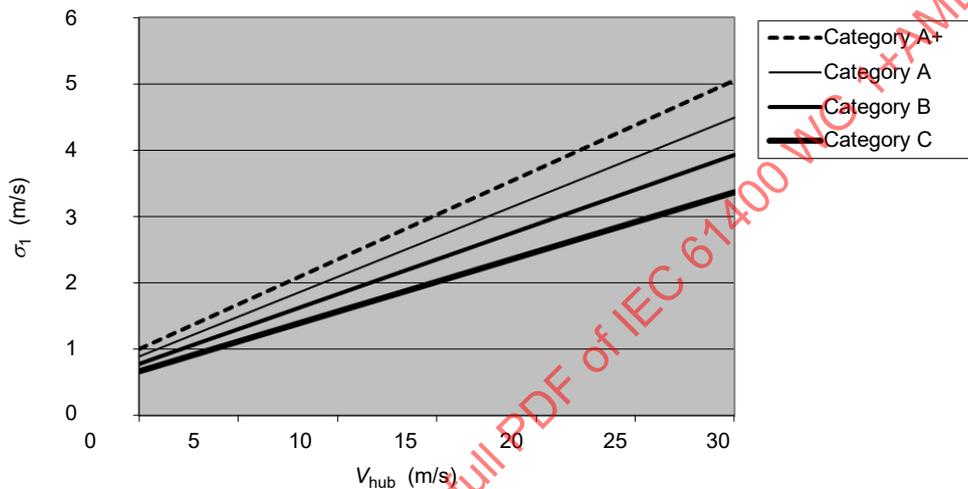
72 where

73
$$k = 0,27 V_{hub} (s / m) + 1,4$$

74
$$C = I_{ref} (0,75V_{hub} + 3,3 \text{ m / s}) \tag{11}$$

75
$$\sigma_1 = I_{ref} (0,75V_{hub} + b); \quad b = 5,6 \text{ m / s} \tag{2}$$

76 Values for the turbulence standard deviation σ_1 and the turbulence intensity σ_1/V_{hub} are shown
77 in Figure 1.

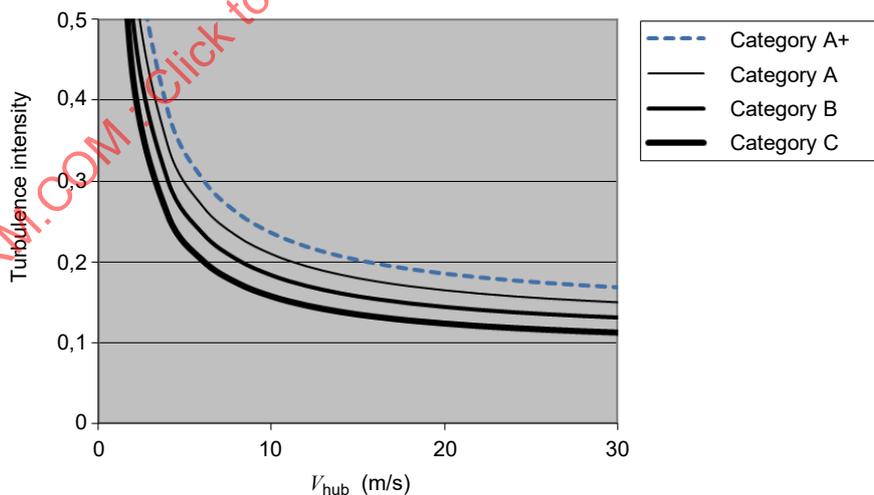


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78

79 **Figure 1a - Turbulence standard deviation**

80



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81

82

Figure 1b - Turbulence intensity

83

Figure 1 - Turbulence standard deviation and turbulence intensity for the normal turbulence model (NTM90 values)

84

85 Values for I_{ref} are given in Table 1.

86 **6.3.3.2 Extreme wind speed model (EWM)**

87 *Replace paragraphs 1 and 2 including eqs.13 and 14 with*

88 The EWM shall be a turbulent wind model. The wind model shall be based on the reference
89 wind speed, V_{ref} , and a fixed turbulence standard deviation, σ_1 . If the wind turbine type is
90 designed for a T class reference wind speed, V_{ref} shall be replaced by $V_{ref,T}$ in the extreme
91 wind speed model while keeping other parameters.

92 *Replace footnote 3 with*

93 3 The turbulence standard deviation for the turbulent extreme wind model is not related to the normal (NTM) or the
94 extreme turbulence model (ETM).

95 **7.4.1 General**

96 *Add after paragraph 5*

97 Serviceability limit states (SLS) consider the function of the structure or one of its components
98 under normal service conditions or the appearance of the structure.

99 Serviceability limit states should be verified with serviceability load levels S1, S2 or S3 as required in
100 relevant IEC 61400 standard or technical specification.

101 For serviceability limit state analyses, S1 is derived from load simulations from the ultimate
102 limit states classified as N (Normal) and for S2 and S3 the same load simulations are used as
103 those used as basis for the fatigue limit state. The partial safety factor for loads shall be: $\gamma_f =$
104 1,0 (1)

105 **7.4.7 Parked (standstill or idling) (DLC 6.1 to 6.4)**

106 *Replace paragraphs 2 and 3 with*

107 For design load cases, where the wind conditions are defined by EWM, the response shall be
108 estimated using either a full dynamic simulation or a quasi-steady analysis with appropriate
109 corrections for gusts and dynamic response using the formulation in ISO 4354. If slippage in
110 the wind turbine yaw system can occur at the characteristic load, the largest possible
111 unfavourable slippage shall be added to the mean yaw misalignment. If the wind turbine has a
112 yaw system where yaw movement is expected in the extreme wind situations (e.g. free yaw,
113 passive yaw or semi-free yaw), the yaw misalignment will be governed by the turbulent wind
114 direction changes and the turbine yaw dynamic response. Also, if the wind turbine is subject to
115 large yaw movements or change of equilibrium during a wind speed increase from normal
116 operation to the extreme situation, this behaviour shall be included in the analysis.

117 In DLC 6.1, for a wind turbine with an active yaw system, a mean yaw misalignment of $\pm 8^\circ$ using
118 the turbulent extreme wind model shall be imposed, provided restraint against slippage in the
119 yaw system can be assured.

120 *Delete paragraph 5: 'The partial safety factors for loads ...'*

121 *Replace paragraphs 6 and 7 with*

122 In DLC 6.3, the extreme wind with a 1-year return period shall be combined with an extreme
123 yaw misalignment. A mean yaw misalignment of $\pm 20^\circ$ using the turbulent wind model shall be
124 assumed.

125 If for the case DLC 6.2, yaw misalignment is evaluated using discrete values, the increment in
126 yaw misalignment shall be not more than 10° in the sector of the maximum lift force on the
127 blades.

128 **7.6.1.3 Partial safety factor for consequences of failure and component classes**

129 *Replace paragraph 1 with*

130 A consequence of failure factor, γ_n , is introduced to distinguish between:

- 131 a. component class 1: used for "fail-safe" components whose failure does not result in the
132 failure of a major part of a wind turbine, for example secondary components and replaceable
133 bearings with monitoring;
- 134 b. component class 2: used for "safe-life" components whose failures may lead to the failure
135 of a major part of a wind turbine;
- 136 c. component class 3: used for "safe-life" components whose failure may lead to human
137 injuries e.g. mechanical components that link actuators and brakes to main structural
138 components for the purpose of implementing non-redundant wind turbine protection
139 functions. Regarding blocking devices, see 7.4.9.

140 *Add before last paragraph*

141 For component class 3, the consequences of failure factor shall be $\gamma_n = 1,2$. If the characteristic
142 value of the load response $F_{gravity}$ due to gravity can be calculated for the design situation in
143 question, and gravity is an unfavourable load, the consequences of failure factor for combined
144 loading from gravity and other sources may have the value

145
$$\gamma_n = 1,1 + 0,1 \zeta^2$$

146
$$\zeta = \begin{cases} 1 - \left| \frac{F_{gravity}}{F_k} \right| & \text{for } |F_{gravity}| \leq |F_k| \\ 0 & \text{for } |F_{gravity}| > |F_k| \end{cases}$$

147 where F_k is the characteristic load.

148

149 **7.6.2.2 Partial safety factors for loads**

150 *Add to paragraph starting with 'When turbulent inflow is used ...'*

151 When the NTM is represented by a statistical distribution (equation 10 & 11) the characteristic
152 value of the load shall correspond to the same return period as obtained using the 90% quantile
153 value NTM90 (equation 12) except for DLC 1.1.

154 **7.6.2.4 Partial safety factors for resistances where recognized design codes are not
155 available**

156 *Delete footnote 17*

157 **7.6.2.5 Partial safety factors for materials where recognized design codes are available**

158 *Replace the clause with*

159 Partial safety factors for resistance, γ_M , shall be applied as given in the recognized design
160 codes, see 7.6.1.4. The partial safety factors for the consequences of failure, γ_n , shall be
161 applied additionally as specified in 7.6.1.3.

162 **7.6.3.4 Partial material factors where recognized design codes are available**

163 *Replace the clause with*

164 Partial safety factors for resistance, γ_M , shall be applied as given in the recognized design
165 codes, see 7.6.1.4. The partial safety factors for the consequences of failure, γ_n , shall be

166 applied additionally as specified in 7.6.1.3. Alternatively, the provisions from section 7.6.3.3
167 may be used.

168 *Add new clause 7.6.7*

169 **7.6.7 Evaluation of limit state through load comparison**

170 As a simplified approach ultimate limit state analysis may be evaluated through a load
171 comparison with a previously analysed design case. The reference loads shall always serve as
172 the reference for a load comparison.

173 This comparison can be used to assess the suitability of an existing structural design for
174 changed environmental conditions (as per clause 11.10) or for minor changes in turbine design
175 (e.g. controller updates or modification in some other turbine components, e.g. tower). In case
176 of a change of a major component the rest of the structure can be assessed based on a load
177 comparison.

178 The following shall be considered as long as no component specific standards within the IEC
179 61400 series specifies otherwise.

180 For extreme loading, a comparison of contemporaneous loads is not required. All mandatory
181 load cases shall be considered. The sign of the extreme load shall be considered when relevant.

182 For fatigue loading, the comparison may be based on DEL's. The slope of the S-N curve shall
183 be in alignment with the design analyses of each component.

184 The effect of mean loads shall be included if relevant.³

185 An exceedance of up to 5% in extreme loads and 3% in fatigue loads compared to the reference
186 loads is acceptable.⁴ In case of exceedances above the given tolerances, all design relevant
187 load signals for the specific component shall be included in the comparison.

188 **7.6.7.1 Rotor Blade**

189 The comparison shall include a subset of blade sections and load directions representative for
190 the design.

191 If there is no relevant data/information available, at least the blade sections blade root, max
192 chord, blade mid and outer third of the blade length and blade flanges shall be considered.

193 **7.6.7.2 Machinery structures and drive train components**

194 For fatigue loading, equivalent loads calculated from the LDD or LRD shall be considered
195 additionally for rotating components like gearboxes, bearings, pitch and yaw systems.

196 **7.6.7.3 Tower and Foundation**

³ If the mean load has a substantial contribution to the fatigue damage and it is driven by the wind speed, and if the mean wind distribution differs significantly from the design conditions, then it is relevant to consider the effect. Especially bolted flange connections may be critical.

⁴ The impact of these tolerances on the COV's for loads and the probability of failure is negligible. In case of safety factor calibration according to Annex K, these tolerances need to be considered as an additional random variable on the load side, with a COV equal to the tolerance.

197 The comparison shall include loads extracted at representative locations of the tower sections,
198 at least the tower top, tower bottom and if applicable at tower sections, which show a significant
199 change in structural properties (e.g. transition sections in hybrid towers).

200 S3 bending moment (S3 limit state as per IEC61400-6) for concrete parts shall also be
201 compared.

202 **7.6.7.4 Blade deflection and tower clearance**

203 The comparison shall include a check of the minimum tower clearance.

204 **10.12 Electro Magnetic Compatibility (EMC)**

205 *Replace paragraph with*

206 **10.12.1 Introduction**

207 This clause describes the design requirements for wind turbines regarding electromagnetic immunity
208 and electromagnetic emissions with respect to Electro Magnetic Compatibility.

209 This includes the design phase evaluation of immunity against EM disturbances generated by EM
210 phenomena of all occurring kind, and the reduction of EM emissions to protect radio services and in
211 general the protection of the electromagnetic spectrum.

212 Furthermore, guidance is given to structure the design documentation in an organized way in Annex
213 N.

214 This documentation, EMC technical documentation, can be (re-)used as a base for final verification
215 and validation purposes which later can be expanded if needed.

216 The local EMC environment is referring to the EMC environment inside the wind Turbine.

217 Site specific EMC environment does vary due to the different geo location(s).

218 The manufacturer can define the wind turbine family definitions used for the designs. When using such
219 family grouping, the EMC properties within a family definition can vary due to design differences. This
220 shall be documented by showing equal EMC performance of the products within the family group.

221 Generic definitions with regards to Electro Magnetic Compatibility:

- 222 • As a minimum, a wind park can consist of 1 turbine + infrastructure. Typically, a wind park
223 consists of multiple wind turbines and their respective infra structure(s).
- 224 • At the highest abstraction level, a wind park can be defined as an (large) installation.
- 225 • The wind turbines used in a wind park are typically industrialized series produced large
226 equipment which can be placed at different locations.
- 227 • Offshore wind parks are in general closed restricted area(s) meaning the number of radio
228 systems which can be disturbed is very small or near zero, and relaxations in the emissions
229 might be possible. (CISPR-H)

230 **10.12.2 EMC design requirements**

231 The wind turbine shall be designed and manufactured in such way that Electro Magnetic Compatibility
232 is ensured. It includes all individual electrical equipment and components containing electronics which
233 are part of the wind turbine as well as the total wind turbine. A holistic approach is required to ensure
234 Electro Magnetic Compatibility.

235 The minimum basic requirements for complete turbine and its constituent components shall be IEC
236 61000-6-2 for immunity and IEC 61000-6-4 for emission.

237 To ensure compatibility of the wind turbine, the manufacturer of wind turbines shall evaluate and
238 document the possible EMC phenomena. If needed, mitigations must be implemented. This can result

239 in additional EMC requirements and mitigations parallel to, or on top of IEC 61000-6-2 + IEC 61000-6-
240 4.

241 When using safety critical or functional safety equipment etc., additional requirements apply.

242 Wireless technology is advancing in the modern world. EMC and other regulatory aspects shall be
243 evaluated when intended radiators and receivers are incorporated into the design of a wind turbine.

244 Possible negatively impacted communication systems due to electromagnetic emissions from the wind
245 turbines and their occurrence shall be evaluated.

246 Electrical sparking is a severe source of EM emissions and possible disturbances as well a known risk
247 factor for fire and generating the corrosive gas Ozon (O₃). With regards to EMC, electrical sparking
248 shall be reduced to the minimum, preferably avoided.

249 The manufacturer has the full responsibility to mitigate these EMC phenomena/effects to ensure
250 compatibility of the wind turbine. Design and production must be aligned to ensure equal level and
251 quality of EMC performance.

252 Detailed EMC documentation of the used equipment and components is required as part of the overall
253 EMC design evaluation. An EMC Technical Documentation shall be compiled to document the design
254 from an EMC perspective⁵.

255 Annex N contains references regarding EMC Technical Documentation, evaluation examples and
256 some guidance for clause 10.12.

257 **11.3.2 Wind condition parameters**

258 *Replace paragraph 5 with*

259 In regions prone to hurricanes⁶, cyclones and typhoons, the extreme wind speed shall be
260 evaluated by appropriate methods, for example as given in Annex J.

261 *To be added to footnote 31*

262 Adjustments of either the load partial safety factors or the extreme 10 min average wind speed, V_{50} ,
263 are optional and not required for compliance with this standard.

264 **11.3.4 Data evaluation**

265 *Replace paragraph 3 with*

266 The average value of the wind speed standard deviation $\hat{\sigma}$, i.e. the standard deviation of the
267 longitudinal turbulence component, and its standard deviation $\hat{\sigma}_{\sigma}$ shall be determined using
268 appropriate statistical techniques applied to measured data⁷.

269 **11.9.2 Assessment of the fatigue load suitability by reference to wind data**

270 *Replace bullet b) with*

⁵ Further EMC requirements and measurement methods are going to be described in the product standard IEC 61400-40. Example: Radiated emissions in the range of 150KHz – 30MHz, 30 MHz – 1 GHz and the according measurement methods for the different frequency ranges used. Advancements regarding in situ emission requirements and methods for high power and large equipment are ongoing in the development of ISM product family standard CISPR 37 and CISPR 11.

⁶ Attention should be given to other wind flow conditions (flow downstream of elevations, low level jets, thunderstorm downburst...) that may be relevant for the turbine loads at the site but had not been considered in the design.

⁷ Linear de-trending may be applied, but unless the trend is included in the load simulation, wind statistic based on de-trended data may lead to underestimation of loads due to missing the long term trend.

- 271 a) An adequate assessment of the ambient turbulence intensity and wake effects can be
272 performed by verifying that the wind speed standard deviation σ_1 from the normal
273 turbulence model (NTM) used in design is greater than or equal to the effective wind speed
274 standard deviation $\hat{\sigma}_{\text{eff}}$ (see Annex E) between the wind speeds V_{ave} and $2V_{\text{ave}}$, i.e.

$$275 \quad \sigma_1 > \hat{\sigma}_{\text{eff}} (= I_{\text{eff}} V_{\text{hub}}) \quad (3)$$

276 If a Weibull distribution is assumed for σ_1 , then the damage equivalent wind speed standard deviation
277 $\sigma_{1,DE}$ from the normal turbulence model (NTM) shall be used instead of σ_1 , with

$$278 \quad \sigma_1 = \sigma_{1,DE} = \left(\sum_i^{N_\sigma} p_i \sigma_{1,i}^m \right)^{\frac{1}{m}}$$

279 where

280 p_i is the probability of the considered turbulence bin, i

281 $\sigma_{1,i}$ is the wind speed standard deviation of the considered turbulence bin, i

282 N_σ is the number of turbulence bins

283 m is the Wöhler curve exponent corresponding to the material of the considered structural
284 component

285 Guidance for calculating $\hat{\sigma}_{\text{eff}}$ can be found in Annex E. In case of complex terrain, the estimated wind
286 speed standard deviation shall be increased in order to account for the distortion of the turbulent flow.
287 This can be done by additional multiplication with a turbulence structure correction parameter CCT as
288 defined in 11.2.

289 *Replace bullet c) with*

- 290 b) The site flow inclination taken as the wind energy weighted mean from all directions, shall
291 be between -8° and $+8^\circ$. Where there are no site data or calculations for the flow inclination,
292 it shall be assumed that the flow inclination is equal to the slope, θ , for the 30° sector within
293 a distance of $5z_{\text{hub}}$ or the $5z_{\text{hub}}$ area extended by $2z_{\text{hub}}$ downwind of the wind turbine
294 position from the wind turbine, see 11.2.

295 *Replace bullet e) with*

- 296 a) The average site air density shall be less than the one specified in 6.4.2 for wind speeds
297 greater than or equal to V_T . As an alternative, for an air density greater than the one specified
298 in 6.4, it shall be demonstrated that the following condition applies:

$$299 \quad \rho_{\text{design}} V_{\text{ave,design}}^2 \cos^2 \phi_{\text{design}} \geq \rho V_{\text{ave,site}}^2 \cos^2 \phi_{\text{site}} \quad (4)$$

300 11.9.3 Assessment of the ultimate load suitability by reference to wind data

301 *Replace bullet b) with*

- 302 b) The site estimate of the extreme 10-min average wind speed V_{50} at hub height with a return
303 period of 50 years shall be less than or equal to V_{ref} adjusted to the site specific air density
304 and site flow inclination. If the only input basis for the site assessment is an extreme 3s
305 average wind speed $V_{50,3s}$ at hub height, the 10-minute average V_{50} may be determined
306 using equation

$$307 \quad V_{50} = \frac{V_{50,3s}}{k_T}$$

308 where $k_T = 1.28$, derived from $k_T = 1 + g_v I_v$ (see ISO 4354), $g_v = 2.56$ and $I_v = \sigma_1 / V_{\text{hub}} = 0.11$,
309 see Equation 17.

310 V50 shall be modified according to Footnote 31 when the coefficient of variation of the
311 annual maximum wind speed is larger than 15 %. For that purpose, it shall be demonstrated
312 that the following condition applies:

$$313 \quad \rho_{design} \cdot V_{ref}^2 \cdot \cos^2 \phi_{design} \geq \rho_{site} \cdot V_{50,hub}^2 \cdot \cos^2 \phi_{site} \quad (5)$$

314 In case of T classification where V_1 is not defined in accordance with eq (15) and (16), in addition to the
315 V_{50} comparison described above, design V_1 shall be evaluated against the corresponding site levels in
316 a similar way.

317 **11.10 Assessment of structural integrity by load calculations with reference to site-** 318 **specific conditions**

319 *Replace paragraphs 1 and 2 with*

320 The demonstration shall comprise a comparison of loads and deflections according to 7.6.7
321 calculated for the specific wind turbine site conditions. The calculations shall account for
322 variations of wind conditions with mean wind direction and speed as well as for wake effects,
323 vertical wind shear, mean wind flow angle, etc.

324 *Replace paragraphs 7 and 8 with*

325 Ultimate limit state analyses shall be performed if one of the criteria in 11.9.3 fails. As a
326 minimum, turbulence (NTM and ETM) and extreme wind conditions (EWM) shall be evaluated,
327 for which the following ultimate design load cases shall be assessed: DLC 1.1, DLC 1.3,
328 DLC 6.1, and DLC 6.2. If the design load cases are covering the different operational events
329 characteristic for the site of interest, no further evaluations need to be performed.

330 Annex B provides definitions of the aforementioned ultimate and fatigue load cases for site-
331 specific conditions. In case of wind turbines located at sites subjected to cold climate conditions,
332 earthquakes, wake effects or LVRT exceeding the design conditions, the corresponding
333 relevant load cases DLC1.6, DLC1.7, DLC1.8, DLC2.5, DLC6.5, DLC6.6 or DLC6.7 in Table B.1
334 should be considered. Other design situations 2), 3), 4), 5), 7) and 8) in Table B.1 only need to
335 be considered when the control system behaviour and transport, assembly, maintenance and
336 repair procedures are site-dependent.

337 For turbines designed with standard classes in Clause 6 the load cases using ECD, EDC, EOG
338 and EWS do not need to be considered in the site assessment.

339 For Class S designs, the load cases using ECD, EDC, EOG and EWS do not need to be
340 considered in the site assessment if those load cases were considered for the design with air
341 density, wind shear and flow angle values of the standard classes in clause 6, and the value of
342 σ_1 for the gusts used in these design load cases was equal or larger to the value of σ_1 for the
343 ambient NTM (non-wake conditions) used for the design⁸.

344 **Annex B**

345 **B.1 General**

346 *Replace paragraph 6 with*

⁸ If ECD, EDC, EOG and EWS are to be considered in the site assessment, they may be considered with the standard class values in clause 6 of air density, wind shear and flow angle, and site specific representative ambient turbulence if no further information of gusts at the site is available.

347 As stated in 11.10 for site suitability assessment, site suitability assessment as a minimum turbulence
348 (NTM and ETM) and extreme wind conditions (EWM) shall be evaluated, for which the following
349 ultimate design load cases shall be assessed: DLC 1.1, DLC 1.3, DLC 6.1, and DLC 6.2. If the design
350 load cases are covering the different operational events characteristic for the site of interest, no further
351 evaluations need to be performed. In case of wind turbines located at sites subjected to cold climate
352 conditions, earthquakes, wake effects or LVRT exceeding the design conditions, the corresponding
353 relevant load cases DLC1.6, DLC1.7, DLC1.8, DLC2.5, DLC6.5, DLC6.6 or DLC6.7 in Table B.1
354 should also be considered in the assessment.

355 Other design situations 2), 3), 4), 5), 7) and 8) in Table B.1 only need to be considered when the
356 control system behaviour and transport, assembly, maintenance and repair procedures are site-
357 dependent.

358 Load cases using ECD, EDC, EOG and EWS in Table B.1 are only relevant for design purposes of
359 Class-S wind turbines.

360 **B.2 Power production (DLC 1.1 to 1.9)**

361 *In table B.1 reference to footnote 25 in the abbreviation ETM_s with reference to footnote 32.*

362 *Replace paragraph 5 with*

363 In DLC 1.6, the assessment of loads resulting from the ambient site-specific ETM_s model in
364 combination with an inter-turbine wake condition shall be assessed. The combined probability
365 of ambient turbulence and waked turbulence shall have a return period of at least 50 years. The
366 inter-turbine spacing S shall cover the worst loading conditions between a minimum spacing
367 S_{\min} (to be defined by the manufacturer) and $S = 20D$, where D is the turbine rotor diameter.

368 Alternatively, wake effects may be considered in the evaluation of DLC 1.1 in which case DLC
369 1.6 may be omitted.

370 **Annex E**

371 *Add the following below formula E.3 and the list of variables:*

372 Consistent with 6.3.2.3, the effective wind speed standard deviation might alternatively be calculated
373 using the site-specific Weibull distribution of the ambient turbulence. In that case the representative
374 turbulence in equation (E.3), $\hat{\sigma}_c$, should be calculated from the following formula,

$$\hat{\sigma}_c = \left(\sum_i^{N_\sigma} p_i \sigma_i^m \right)^{\frac{1}{m}} \quad (\text{E.XX})$$

375 where

376 p_i is the probability of the considered turbulence bin, i

377 σ_i is the wind speed standard deviation of the considered turbulence bin, i

378 N_σ is the number of turbulence bins

379 If the representative turbulence, $\hat{\sigma}_c$, is calculated from equation (E.XX) then, the corresponding
380 effective turbulence, I_{eff} , found from equation (E.3) must not be used for load calculations, it may only
381 be used for comparison of damage equivalent turbulence intensities.